APPLICABLE BUILDING CODES ALL NEW WORK SHALL COMPLY AND CONFORM TO THE REQUIREMENTS OF THE 2013 CBC CLASS LEASING, LLC. 1221 Harley Knox Blvd. Perris, CA 92571-7408 Fax (951) 943-5768 (951) 943-1908 -2013 CALIFORNIA ENERGY CODE (CEC) PART 6, TITLE 24, CCR* -2013 CALIFORNIA FIRE CODE PART 9, TITLE 24, CCR SPECIFICATIONS RELOCATABLE CLASSROOMS 1.01 GENERAL REQUIREMENTS TITLE 19 CCR PUBLIC SAFETY, STATE FIRE MARSHALL REGULATIONS 1. The requirements of the general conditions of the agreement and these General Requirements apply to the several trade sections with the same force as though fully repeated in each section. a) FRAMING: securely nailed, bridged and blocked to form rigid structure. Work cut, fitted and assembled level. Name brands are indicated to establish a standard of quality. Items of equal or better quality may be substituted for the listed brand named products. 1:02 DESIGN DATA: SCOPE OF WORK: 1. The work consists of installing on-site, modular Relocatable buildings as defined herein, shown and detailed on c) Machine applied nalling shall have prior demonstration and approval by DSA Field Inspector and the Architect. The approval is subject to continuous satisfactory performance. Plywood shall have a minimum thickness of 3/8". FLOOR LIVE LOAD = 50 PSF, 50 + 20 PSF PARTITIONS, 100 PSF ROOF LIVE LOAD = 20 PSF REDUCIBLE FOR TRIBUTARY AREA the drawings. 2. All requirements of CCR (California Code of Regulation) Title 19 and 24 relating to inspections and verified reports

1.03

1.04

2.01

WORK NOT INCLUDED;

SITE ASSEMBLY:

SAMPLE: FOR WELD PLATE OPTION ONLY

ORTANT: This form is only a summary list of structural tests a

itural components, etc., per Title 2 de for projects submitted for review

STRUCTURAL STEEL AND COL

e. Verily WPS, welder qualifications and equipment

19.2 FIELD WELDING:

single-pass filet wolds < 5/15"

used for the project. The actual lasts and inspections, cruit i

+ SOILS

+ ICONCRETE

+ IMASONRY

+ OTHER

and the District Architect.

Bat AN Sel Det

3.01 CARPENTRY:

1. Scope of Work: Contractor shall provide all labor, materials and services to install carpentry

- plumb and true to line. Trim in as long lengths as possible with all standing trim in one piece. Trim sealed at all edges.
- b) NAILING: in accordance with the title 24 CCR-Table 2304.9.1. Nails shall be corrosion resistant box nails.
- If nail heads penetrate the outer ply more than would be normal for a hand hammer or if minimum allowable edge distances are not maintained, the performance will be deemed unsatisfactory.
- d) TRIM: sealed at all edges. Sealant painted to match trim or siding

4.01 MATERIAL SPECIFICATIONS:

- 1. Structural framing shall be Hem Fir- Larch graded in accordance with the standard grading rules of the Western Wood Products Association or standard grading rules No. 16 of the West Coast Lumber Inspection Bureau, latest editions. Grades shall be as follows unless noted otherwise on the drawings. (Hem Fir South is <u>not</u> allowed.) Each piece shall be grade marked and no piece may fall below grades indicated. All framing except as noted Hem Fir No. 2
- 2. Plywood shall be as shown on these drawings with exterior glue in accordance with U.S. Product Standard PS 1-07. All panels shall be marked with an APA grade mark with an identification index as shown on drawings. Use 4'x8' panelsminimum, except at boundaries and at framing changes where minimum panel dimension shall be 24" at roofs and floors and 12" at walls.
- 3. Bolts for timber connections shall conform to ANSI/ASME Standard B18.2.1-2012 & 2012 edition of NDS (the National Design Specification for Wood Construction by the National Forest Products Association). Bolts shall be installed in accordance with the requirement of 2012 NDS. Bolt holes shall be 1/32 to 1/16 inch larger than bold diameter. Bolts shall be full body steel bolts with minimum yield strength of 45,000 PSI. Re-tighten bolts before closing in work.
- 4. Lag screws shall be steel and conform to ANSI/ASME Standard B18.2.1 and 2012 NDS. Holes for lag screw shanks shall be bored the same depth and diameter as the shank. The remaining depth of penetration of the screw shall be bored to 70% of the shank diameter. One quarter inch (1/4") d'ameter lag screws need not have pre-drilled holes if it can be shown that wood members are not demaged during installation. Provide full diameter body lag screws with bending yield strengths per Table 11J in NDS
- 5. Provide malleable iron washers or equivalent cut plate washers (not less than a standard cut washer) under nuts and f bolt or lag screw heads which bear on wood.
- 6. Wood screws shall conform to ANSI/ASME Standard B18.6.1 and the requirements of the 2012 NDS. Galvanized or other corrosion resistant coating where exposed to weather or used in foundations Screws shall be steel with cut threads and bending yield strengths per Table 11L in NDS.
- 7. Wood members shall be cut or notched only as shown on structural drawings.
- 8. When required nailing tends to split wood members, nail holes shall be pre-bored to 3/4 of the nail diameter.
- 9. Structural nailing shall be with BOX NAILS per all requirements of 2012 NDS. Nailing not specifically indicated shall comply with CCR Title 24, Part 2, Table 2304.9.1. All nails shall be galvanized or other corrosion resistant coating where exposed to weather, in foundations and as noted on plans, per the requirements of CCR Title 24, Part 2, with minimum ending yields per table 11N in NDS. (See nail equivalence below.)
- 10. Nail equivalence: (provide minimum nail lengths as required for specified penetration, TYPICAL: U.N.O.)

6d equals .113" DIA. - provide 1.36" minimum point penetratio 8d equals .131" DIA. - provide *1.57" minimum point penetration

- 11. Pressure preservative treatment shall be per Section 2303.1.8, CCR Title 24, Part 2. Provide quality mark on all treated undation members that comply with CBC 2303.1.8.1. All foundation members shall be marked as "For ground contact or "For above ground use" as appropriate. Pressure treated material shall comply with AWPA Standard U1 as required by CBC 2303.1.8. Treat all cut ends of pressure treated members with an approved preservative. (Willard W/B Copper Green 2% or an approved equivalent). Where noted, members below the sub floor that are not a part of the foundation shall be pressure treated.
- 12. Only material in contact with ground needs to be pressure treated, all other foundation lumber can be DF or HF#2 or
- 13. If machine nailing is utilized for this project, contractor shall comply with all requirements of CCR Title 24, Part 2. Machine nailing is subject to approval by the Structural Engineer or Architect and the Division of the State Architect.
- 14. Fasteners for pressure-preservative treated and fire-retardant treated wood shall comply with Section 2304.9 of CBC.
- 15. Nails and spikes used in wet or exterior locations shall comply with Section 2304.9.1.1 of CBC.
- 16. Shim material shall be plywood CD EXP 1 or equal (not pressure treated).
- 17. Used lumber in good condition is acceptable for use in foundation system.
- 18. Tie plates shall conform to A-1011 Grade 33.

5.01 SITE INSTALLATION REQUIREMENTS FOR DSA CLASSROOM BUILDINGS:

In the case of equipment located in the State of California, the LESSEE (School District) is responsible for the site being cleared (free of grass, trees, shrubs, etc) and graded to within 4 1/2" of level grade for each building. If the site exceeds the 4 1/2" level grade requirement additional costs may be charged to lessee.

Under no circumstances should the site be greater that 9" from level grade or have less than a 1000 PSF MINIMUM SOIL BEARING PRESSURE.

Prior to delivery, the lessee shall mark the four comers of the building on the site, including door location. Should special handling be required to either place, install or relocate the classroom on the lessee's site due to site obstruction such as fencing, landscaping, other classrooms, etc., additional costs will be charge to the lessee.

6.01 TEST AND INSTALLATION:

TOPURBATTING FORM NO. 181-

- 1. Provide Electrical Grounding Test per DSA IR E-1.
- 2. Field Welding for welded tie plate option. (If used, requires Test and Inspection.)

The example form DSA 103's shown on this sheet are for illustration purposes only. A form DSA 103 is to be completed for each application that this PC is being incorporated into and all example form DSA-103's are to be crossed out on this drawing.

3. No other tests and inspections are required.

shall be complied with and shall include a) General responsible charge of Field Administration by the Architect of Record.

b). Inspection during the course of construction by an inspector approved by DSA (Division of the State Architect) and the District Architect. The Inspector shall be responsible for and approved to inspect the general construction, welding, mechanical and electrical work. Cost of these inspections shall be borne by the School

c) On site inspection of the building installation, electrical and utility of the building installation or connection by an Inspector approved by the DSA and retained by the School District. d) Other special tests or inspections as may be required by DSA. Cost of these inspections/tests shall be borne by the School District

1. All on-site or off-site utilities and the connection of them to the building unless indicated on the drawings. All leveling, grading or other site preparation (except concrete or wood leveling strips, where Required)
unless otherwise indicated on the drawings.

3. Fire alarm system, program bell, clock, public address system, intercom system, TV system, computer data or any other low voltage system, unless otherwise indicated on the drawings or the lease agreement.

ACCESSIBILITY OF SITE: The School District shall provide access to the site for the installation of the building. Removal of trees, shrubs, fencing, sprinklers, etc. necessary for move in and removal of the buildings shall be the responsibility of the School District.

Scope of Work: Contractor shall provide all labor, materials and services to prepare the building elements, transport them from the plant to the site and to complete the assembly at the site. The condition of the site, such as drainage and soil bearing capacity, shall be the responsibility of the School District

2. Assembly of Elements: a) In a location on the site as determined by the District Architect. The contractor shall place the foundation as letailed on the drawings. b) The elements shall be brought to the site on wheel assembly and transferred to the prepared site. Great care

shall be taken to avoid damage to the elements by racking or bumping. c) Connection of the elements together shall be done according to instructions on the drawings. Flashing, trim and morine even we morened for highs citie details of the diff

DSA-103 aw 132013		ing Anglandari Anglandari	HOREMENTS DSA File No.:	
Statement of Struc Special Inspection			Date Submitted:	
TE OPTION ONLY	ſ	District	Revised: L	
many list of structural tests and specie	Inspections	WST	RECTIONS: Click a plus algo (+) before any category or subcategory to reveal additional and special inspections. An "X" before a listed jest or inspection indicates it is a mandatory	
to and inspections insult be performen oper inspector is responsible for provi but not inside to, special inspections ing, high-load wood dispirages, cold components, etc., per Tille 24, Pari 2, projects submitted for review under the	ding inspection not listed on formed steel Chapter 17A.	requi depi indic indic Cilci	imment. A shaded box indicates a test or appoint inspection that may be required, inding on the ecope of the construction and other issues. A shaded box can be olicked adap your selection of that test. Noise A minus (-) on a category or subcategory heading also that it can be colleged. However, sity selections you may have made will be cloared. (on the "COMPILE" button to show only the tests finally selected. For more information or of this form, see DSA-103.MBTR.	
Note: References are to the 201	S edition of the C	tifornis Bi	licing Code (CBC) unless otherwise noted.]
ITION	1. AND		0 OODE REPERENCE AND NOTES	
	Table 1708A.3			
	THE 402-11/AC	1630-11/A	ICE 5-11 Tuble 1.19.3	
	Table 1704A.2.	1		
AL STEEL AND COLD-FORM	IED STEEL L	ISED FO	DR STRUCTURAL PURPOSES	·····
ppropriately marked and that: Marial proparties that comply with	Periodic		* By special inspector when performed off-site; by project inspector for steel shipped directly to project site without welding or fabrication.	
rades comply with requirements.	Test	طعا	12283A.1 (2203.1 [*]). ASTM AS70.	-
sting and all delaits constructed in	Continuous	- PI		
nnection lab locations and all Id in the shop.	Periodic	81		
			DSA IR 17-S, AWS D1.1 and AWS D1.8 (AWS D1.8 for cold formed sleet).	-
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ntilication matrices per AWS A approved documents and the WPS. Invited wer's conflicate of				4 1 1927
ilions and equipment.	Periodic Periodic	81 81	DSA iR 17-3.	
MELDING:			LOSS IN 17-5.	
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linuous special inspection is required		autorized	representative	5
a speciel inspection is required		Lab - indi	inter that the last or inspection is to be performed by a testing laboratory accepted in the DSA Evolution and Acceptance (LEA) Program. See section 4-596, 2015 CCR Tills 24, Part 1.	
ad		Free contraction of the local division of the local division of the local division of the local division of the	tes that the special inspection is to be performed by the project inspector	
	-	81 - Indice	tes that the special inspection is to be performed by a special inspector	
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2013 CALIFORNIA CODE OF REGULATIONS (CCR) As of January 01, 2014* -2013 CALIFORNIA BUILDING STANDARDS ADMINISTRATIVE CODE PART 1. TITLE 2 (2012 INTERNATIONAL BUILDING CODE VOLUMES 1-2 WITH 2013 CALIFORNIA AMENDMENTS)

-2013 CALIFORNIA ELECTRICAL CODE (CEC) PART 3, TITLE 24, CCR (2011 NATIONAL ELECTRICAL CODE WITH 2013 CALIFORNIA AMENDMENTS) -2013 CALIFORNIA MECHANICAL CODE (CMC) PART 4, TITLE 24, CCR (2012 UNIFORM MECHANICAL CODE WITH 2013 CALIFORNIA AMENDMENTS)

-2013 CALIFORNIA PLUMBING CODE (CPC) PART 5, TITLE 24, CCR (2012 UNIFORM PLUMBING CODE WITH 2013 CALIFORNIA AMENDMENTS)

(2012 INTERNATIONAL FIRE CODE WITH 2013 CALIFORNIA AMENDMENTS) -2013 CALIFORNIA REFERENCED STANDARDS CODE PART 12, TITLE 24, CCR

WIND SPEED =120 MPH (V) (3 SECOND GUST). K zT = 1.0 SNOW LOAD: PROJECT IS NOT LOCATED IN A SNOW REGION. BUILDING CODES = 2012 IBC AND CBC 2013

ROOF LEVEL

-FINISH FLOOR

BUILDING CODES = 2012 IBC AND CBC 2013		
ISMIC DESIGN DATA: MOMENT FRAME PC'S sic Seismic-Force-Resisting System = STEEL MOMENT FRAME ALYSIS PROCEDURE USED = EQUIVALENT LATERAL FORCE Ismic Design Category = E (per CBC Section 16'13A.6.6) sign Base Shear : 24x40 BUILDING = 9460 # (Roof, Floor, Walls & Partitions) 36x40 BUILDING = 14190 # (Roof, Floor, Walls & Partitions) 46x40 BUILDING = 18920 # (Roof, Floor, Walls & Partitions)	Design Base Shear: 24x40 BUILDING 36x40 BUILDING	= EQUIVALENT LATERAL = E (per CBC Section 161
 Ca2 = 0.411 R: = 3.5 SITE CLASS = D To apped value / 0.8 Se = 2.16 (For Design) S = 1.44 (Site Specific Documentation Justifying SDS Shall Be Submitted To DSA Prior To Approval) String SDS Shall Be Submitted To DSA Prior To Approval) K CATEGORY = II FLOOD DESIGN DATA: Project is not located in a flood zone. 	S1 = 1.3 per CBC Figure 1813A.6(2) RISK CATEGORY = 11	16 (For Design)
LIMITATIONS FOUNDATION PC ONLY:		
FOUNDATION ONLY PC IS DESIGNED TO SUPPORT THE SUPERSTRUCTURE FOR THE RELOCATABLE BUILDINGS AS LISTED ON THIS DRAWING.		•
THE DESIGN CALCULATIONS ARE BASED ON THE FOLLOWING:		
1. DSA APPROVED STOCKPILE BUILDINGS		
2. ROOF OVERHANGS OF 5'-0" MAXIMUM		
3. SINGLE SLOPE OR DUAL SLOPE BUILDINGS WALL HEIGHT: 9'-0' MAXIMUM ON DUAL SLOPE BUILDING. WALL HEIGHT: 10'-4" MAXIMUM ON SINGLE SLOPE BUILDING. (HEIGHT DETERMINED FROM FINISH FLOOR IN BUILDING TO BOTTOM OF STEEL ROOF STRUCTURE: BEAMS OR ROOF HE/ WALL HEIGHT: 9'-10" MAXIMUM ON SHEAR WALL-DUAL SLOP		
4. WALL DEAD LOAD OF 10 PSF (NO STUCCO)		•
5. FLOOR DEAD LOAD OF 8 PSF	· · ·	• ,
ROOF SLOPE	ROO	SLOPE
LO MAX.	6	1 10.3 1/2"
BOTTOM OF ROOF DEAD LOAD CANNOT HEADER OR BEAM EXCEED 18 PSF	BTH OF STL ROOF RAFTERS BOTTOM OF ROO RAFTER-2×6	EXCEED 10 PSF
FINISH FLOOR PLOOR DEAD LOAD OD CANNOT EXCEED 8 PSF	FINISH FLOOR	FLOOR DEAD LOAD CANNOT EXCEED 8
SHEAR WALL-DUAL SLOPE PC-247	SHEAR WALL- TYPICAL	DUAL SLOPE

OVERHANG MAX. EXTERIOR VALL -BOTTOM OF ROOF DEAD LOAD CANNOT HEADER/BEAM OR RAFTER FRONT BACK EXCEED 10 PSF FLOOR DEAD LOAD -FINISH FLOOR CANNOT EXCEED 8 PSF GRADE 40'-0" SIDE WALL SHEAR WALL-SIDE WALL 1/8"=1'-0" OVERHANG OVERHANG 5'MAX. ROOF SLOPE 5'0" ROOF LEVEL totas EXTERIOR WALL -BOTTOM OF ROOF DEAD LOAD CANNOT HEADER OR BEAM FRONT BACK EXCEED 10 PSF FLOOR DEAD LOAD -FINISH FLOOR CANNOT EXCEED 8 PSF -GRADE 40'-0" SIDE WALL PC104-1/8"=1'-0" DUAL SLOPE ROOF OVERHANG OVERHANG MAX. ROOF SLOPE 5'MAX. <u>, 5'0"</u> ROOF LEVEL L EXTERIOR VALL -BOTTOM OF ROOF DEAD LOAD CANNOT HEADER OR BEAM FRONT EXCEED 10 PSF

TYPICAL ELEVATIONS ARE SHOWN TO CLARIFY FOUNDATION PC ONLY LIMITATIONS DOCUMENTATION SHALL BE PROVIDED BY ENGINEER OF GENERAL RESPONSIBLE CHARGE TO BE REVIEWED AND APPROVED BY THE DSA STRUCTURAL PLAN REVIEWER.

40'-0" SIDE WALL

MONO SLOPE ROOF

FLOOR DEAD LOAD

CANNOT EXCEED & PSF

-GRADE

1/8"=1'-0"

